Location : BH12

Pile Length .(M)	10.000	If Liquified strata than liquified level	No
Pile cutoff Length (m)	1.000	Pile Terminating level	
Angle of Internal Friction at Pile Tip (Degree)	Ø	31	
Cohesion at Pile tip (t/m2)	c	0	

Bearing Capacity Factor			
Nc	Nq	Ny	
9	25	27.52	31

Ultimate pile capacity , Qu = QP + Qs = { Ap Nc Cp + Ap (1/2 D γ Nγ + P_D Nq} +{ Sum (K P_{Di} tan d Asi) + alpha x Ca As }
Qp = End bearing resistnce , Qs = Frictional resistsnce

1 Pile dia (m) = 0.45

Depth from	Depth to	Length below cutoff (m)	Thickness of sand layer	Li	C, T/m2	alpha	Pile dia D, m	Circumferential area As, m2	K	phi	Y (metric ton/m3)	Ysub (metric ton/m3)	po' ton/m2	Qs, Tonne	Qp, Tonne	Qu, Tonne	Qsafe in copression , Metric Tonne	Q safe Uplift, Metric Tonne
0.00	2.50			2.50	Ignored						0.0							
2.50	9.60			7.10	3.33	0.78	0.45	10.04		0	1.75	0.75	5.33	26.07				
9.60	10.00		0.40	0.40			0.45	0.57	1.00	31	1.80	0.80	5.49	1.86				
15D =	6.75	9.00	0.40	6.75			0.45			0	1.85	0.85	5.74	27.93	23.65	51.58	20.63	10.13

2 Pile dia (m) = 0.5

0.00	2.50			2.50	Ignored						0.0							
2.50	9.60			7.10	3.33	0.78	0.50	11.15		0	1.75	0.75	5.33	28.97				
9.60	10.00		0.40	0.40			0.50	0.63	1.00	31	1.80	0.80	5.49	2.07				
15D =	7.50	9.00	0.40	7.50			0.50			0	1.85	0.85	6.38	31.04	32.44	63.48	25.39	11.52

3 Pile dia (m) = 0.6

0.00	2.50			2.50	Ignored						0.0							
2.50	9.60			7.10	3.33	0.78	0.60	13.38		0	1.75	0.75	5.33	34.76				
9.60	10.00		0.40	0.40			0.60	0.75	1.00	31	1.80	0.80	5.49	2.48				
15D =	9.00	9.00	0.40	9.00			0.60			0	1.85	0.85	7.65	37.25	56.06	93.31	37.32	14.46

Location : BH13

Pile Length .(M)	9.000	If Liquified strata than liquified level	No
Pile cutoff Length (m)	1.000	Pile Terminating level	
Angle of Internal Friction at Pile Tip (Degree)	Ø	0	
Cohesion at Pile tip (t/m2)	c	6.7	

Bearing Capacity Factor		
Nc	Nq	Ny
9	0.00	0.00

Ultimate pile capacity , Qu = QP + Qs = { Ap Nc Cp + Ap (1/2 D γ Nγ + P_D Nq} +{ Sum (K P_{Di} tan d Asi) + alpha x Ca As }
Qp = End bearing resistnce , Qs = Frictional resistsnce

1 Pile dia (m) = 0.45

Depth from	Depth to	Length below cutoff (m)	Thickness of sand layer	Li	Cav, T/m2	alpha	Pile dia D, m	Circumferential area As, m2	K	phi	Y (metric ton/m3)	Ysub (metric ton/m3)	po' ton/m2	Qs, Tonne	Qp, Tonne	Qu, Tonne	Qsafe in copression , Metric Tonne	Q safe Uplift, Metric Tonne
0.00	2.00			2.00	Ignored													
2.00	9.00			7.00	4.46	0.58	0.45	9.90						25.60				
		8.00					0.45							25.60	9.59	35.19	14.08	9.22

2 Pile dia (m) = 0.5

0.00	2.00			2.00	Ignored													
2.00	9.00			7.00	4.46	0.58	0.50	11.00						28.44				
		8.00					0.50							28.44	11.84	40.28	16.11	10.48

3 Pile dia (m) = 0.6

0.00	2.00			2.00	Ignored													
2.00	9.00			7.00	4.46	0.58	0.60	13.19						34.13				
		8.00					0.60							34.13	17.05	51.18	20.47	13.14

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6.12

Location: BH14

Pile Length .(M)	14.000	If Liquified strata than liquified level	No
Pile cutoff Length (m)	1.000	Pile Terminating level	
Angle of Internal Friction at Pile Tip (Degree)	Ø	0	
Cohesion at Pile tip (t/m2)	c	3.3	

Bearing Capacity Factor		
Nc	Nq	Ny
9	0.00	0.00

Ultimate pile capacity , Qu = QP + Qs = { Ap Nc Cp + Ap (1/2 D γ Nγ + PD Nq} +{ Sum (K PDi tan d Asi) + alpha x Ca As }
Qp = End bearing resistnce , Qs = Frictional resistsnce

1 Pile dia (m) = 0.45

Depth from	Depth to	Length below cutoff (m)	Thickness of sand layer	Li	Cav, T/m2	alpha	Pile dia D, m	Circumferential area As, m2	K	phi	Y (metric ton/m3)	Ysub (metric ton/m3)	po' ton/m2	Qs, Tonne	Qp, Tonne	Qu, Tonne	Qsafe in copression , Metric Tonne	Q safe Uplift, Metric Tonne
0.00	2.00			2.00	Ignored													
2.00	14.00			12.00	2.66	1.00	0.45	16.96						45.13				
		13.00					0.45							45.13	4.72	49.85	19.94	15.99

2 Pile dia (m) = 0.5

0.00	2.00			2.00	Ignored													
2.00	14.00			12.00	2.66	1.00	0.50	18.85						50.14				
		13.00					0.50							50.14	5.83	55.97	22.39	18.15

3 Pile dia (m) = 0.6

0.00	2.00			2.00	Ignored													
2.00	14.00			12.00	2.66	1.00	0.60	22.62						60.17				
		13.00					0.60							60.17	8.40	68.57	27.43	22.70

6-13

Location: BH15

Pile Length .(M)	8.000	If Liquified strata than liquified level	No
Pile cutoff Length (m)	1.000	Pile Terminating level	
Angle of Internal Friction at Pile Tip (Degree)	Ø	31	
Cohesion at Pile tip (t/m2)	c	0	

Bearing Capacity Factor			
Nc	Nq	Ny	
9	25	27.52	31

Ultimate pile capacity , Qu = QP + Qs = { Ap Nc Cp + Ap (1/2 D γ Nγ + P_D Nq} +{ Sum (K P_{Di} tan d Asi) + alpha x Ca As }
Qp = End bearing resistnce , Qs = Frictional resistsnce

1 Pile dia (m) = 0.45

Depth from	Depth to	Length below cutoff (m)	Thickness of sand layer	Li	C, T/m2	alpha	Pile dia D, m	Circumferential area As, m2	K	phi	Y (metric ton/m3)	Ysub (metric ton/m3)	po' ton/m2	Qs, Tonne	Qp, Tonne	Qu, Tonne	Qsafe in copression , Metric Tonne	Q safe Uplift, Metric Tonne
0.00	2.50			2.50	Ignored						0.0							
2.50	6.80			4.30	3.56	0.78	0.45	6.08		0	1.75	0.75	3.23	16.88				
6.80	8.00		1.20	1.20			0.45	1.70	1.00	31	1.80	0.80	3.71	3.78				
15D =	6.75	7.00	1.20	6.75			0.45			0	1.85	0.85	5.74	20.66	23.65	44.31	17.72	7.57

2 Pile dia (m) = 0.5

0.00	2.50			2.50	Ignored						0.0							
2.50	6.80			4.30	3.56	0.78	0.50	6.75		0	1.75	0.75	3.23	18.76				
6.80	8.00		1.20	1.20			0.50	1.88	1.00	31	1.80	0.80	3.71	4.20				
15D =	7.50	7.00	1.20	7.50			0.50			0	1.85	0.85	6.38	22.95	32.44	55.39	22.16	8.62

3 Pile dia (m) = 0.6

0.00	2.50			2.50	Ignored						0.0							
2.50	6.80			4.30	3.56	0.78	0.60	8.11		0	1.75	0.75	3.23	22.51				
6.80	8.00		1.20	1.20			0.60	2.26	1.00	31	1.80	0.80	3.71	5.04				
15D =	9.00	7.00	1.20	9.00			0.60			0	1.85	0.85	7.65	27.54	56.06	83.60	33.44	10.84

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Location: BH16

Pile Length .(M)	11.000	If Liquified strata than liquified level	No
Pile cutoff Length (m)	1.000	Pile Terminating level	
Angle of Internal Friction at Pile Tip (Degree)	Ø	0	
Cohesion at Pile tip (t/m2)	c	3.3	

Bearing Capacity Factor		
Nc	Nq	Ny
9	0.00	0.00

Ultimate pile capacity , Qu = QP + Qs = { Ap Nc Cp + Ap (1/2 D γ Nγ + P_D Nq} +{ Sum (K P_{Di} tan d Asi) + alpha x Ca As }
Qp = End bearing resistnce , Qs = Frictional resistsnce

1 Pile dia (m) = 0.45

Depth from	Depth to	Length below cutoff (m)	Thickness of sand layer	Li	Cav, T/m2	alpha	Pile dia D, m	Circumferential area As, m2	K	phi	Y (metric ton/m3)	Ysub (metric ton/m3)	po' ton/m2	Qs, Tonne	Qp, Tonne	Qu, Tonne	Qsafe in copression , Metric Tonne	Q safe Uplift, Metric Tonne
0.00	2.00			2.00	Ignored													
2.00	11.00			9.00	4.86	0.58	0.45	12.72						35.86				
		10.00					0.45							35.86	4.72	40.59	16.24	12.63

2 Pile dia (m) = 0.5

0.00	2.00			2.00	Ignored													
2.00	11.00			9.00	4.86	0.58	0.50	14.14						39.85				
		10.00					0.50							39.85	5.83	45.68	18.27	14.33

3 Pile dia (m) = 0.6

0.00	2.00			2.00	Ignored													
2.00	11.00			9.00	4.86	0.58	0.60	16.96						47.82				
		10.00					0.60							47.82	8.40	56.22	22.49	17.90

Ref : Appendix-C (cl 6.5.2) of IS 2911 (Part 1/Sec. 2) – 2010

Stiffness factor $R = \sqrt[4]{\frac{EI}{KB}}$ for clay soil

$T = \sqrt[5]{\frac{EI}{\eta h}}$ for sandy soil

E = Modulus of Elasticity of pile material= $5000\sqrt{f_{ck}}$

E = 25×10^6 KN/m^2 for concrete for f_{ck} = 25 N/mm^2

I = Moment of Inertia = $\pi D^4/64$

B= D = diameter of pile

Deflection of pile

$$y = \frac{H(e+Zf)\times 1000}{12\,EI}$$

H= lateral load in KN

y = deflection of pile head in mm

e = cantilever length above ground/ bed

E= Modulus of elasticity in KN/m^2

I = Moment of Inertia in m^4

Z_f= Depth of point of fixity in m

Calculation Details (Clay)

		fck=	25	N/mm2	
		E=	25000000	KN/m2	
1	Pile dia	B=D(m)	0.45		
2	Pile Length	L(m)	8.00		
3	Pile Length (soft soil)	L1(m)	0.00		
					for Cohesive soil
5	k1= Modulus of subgrade reaction		19500.00	KN/m3	(medium stiff)
6	K = k1x0.3/(1.5 xB)		8666.67		
7	I= Moment of inertia		0.00	m4	
8	E = Modulus of elasticity		25000000	KN/m2	
9	R		1.90		
10	L1/R		0.00		
11	2R		3.79		L=12 > 2R
12	3.5R				
13	Lf/R		2.00		
14	Lf		3.79		
15	e (m)	eccentricity	0.00		
16		(Length of fixity)	6.68		
17	y=(Permissible deflection mm)		4.50		
18	**H = lateral load capacity**		**49.89**	**KN =**	**4.99** **Ton**

Ref : Appendix-C (cl 6.5.2) of IS 2911 (Part 1/Sec. 2) – 2010

Stiffness factor $R = \sqrt[4]{\frac{EI}{KB}}$ for clay soil

$T = \sqrt[5]{\frac{EI}{\eta h}}$ for sandy soil

E = Modulus of Elasticity of pile material= $5000\sqrt{f_{ck}}$

E = 25×10^6 KN/m^2 for concrete for f_{ck} = 25 N/mm^2

I = Moment of Inertia = $\pi D^4/64$

B= D = diameter of pile

Deflection of pile

$$y = \frac{H(e+Zf)\times 1000}{12\,EI}$$

H= lateral load in KN

y = deflection of pile head in mm

e = cantilever length above ground/ bed

E= Modulus of elasticity in KN/m^2

I = Moment of Inertia in m^4

Z_f = Depth of point of fixity in m

Calculation Details (Clay)

		fck=	25 N/mm2	
		E=	25000000 KN/m2	
1	Pile dia	B=D(m)	0.50	
2	Pile Length	L(m)	8.00	
3	Pile Length (soft soil)	L1(m)	0.00	
				for Cohesive soil
5	k1= Modulus of subgrade reaction		19500.00 KN/m3	(medium stiff)
6	K = k1x0.3/(1.5 xB)		7800.00	
7	I= Moment of inertia		0.00 m4	
8	E = Modulus of elasticity		25000000 KN/m2	
9	R		2.11	
10	L1/R		0.00	
11	2R		4.21	L=12 > 2R
12	3.5R			
13	Lf/R		2.00	
14	Lf		4.21	
15	e (m)	eccentricity	0.00	
16		(Length of fixity)	6.68	
17	y=(Permissible deflection mm)		5.00	
18	**H = lateral load capacity**		**61.59 KN =**	**6.16 Ton**

Ref : Appendix-C (cl 6.5.2) of IS 2911 (Part 1/Sec. 2) – 2010

Stiffness factor $R = \sqrt[4]{\frac{EI}{KB}}$ for clay soil

$T = \sqrt[5]{\frac{EI}{\eta h}}$ for sandy soil

E = Modulus of Elasticity of pile material= $5000\sqrt{f_{ck}}$

E = 25×10^6 KN/m^2 for concrete for f_{ck} = 25 N/mm^2

I = Moment of Inertia = $\pi D^4/64$

B= D = diameter of pile

Deflection of pile

$$y = \frac{H(e+Zf)\times 1000}{12\,EI}$$

H= lateral load in KN

y = deflection of pile head in mm

e = cantilever length above ground/ bed

E= Modulus of elasticity in KN/m^2

I = Moment of Inertia in m^4

Z_f = Depth of point of fixity in m

Calculation Details (Clay)

		fck=	25	N/mm2	
		E=	25000000	KN/m2	
1	Pile dia	B=D(m)	0.60		
2	Pile Length	L(m)	8.00		
3	Pile Length (soft soil)	L1(m)	0.00		
					for Cohesive soil
5	k1= Modulus of subgrade reaction		19500.00	KN/m3	(medium stiff)
6	K = k1x0.3/(1.5 xB)		6500.00		
7	I= Moment of inertia		0.01	m4	
8	E = Modulus of elasticity		25000000	KN/m2	
9	R		2.53		
10	L1/R		0.00		
11	2R		5.05		L=12 > 2R
12	3.5R				
13	Lf/R		2.00		
14	Lf		5.05		
15	e (m)	eccentricity	0.00		
16		(Length of fixity)	6.68		
17	y=(Permissible deflection mm)		6.00		
18	**H = lateral load capacity**		**88.69**	**KN =**	**8.87** **Ton**

LOCATION -BH1

LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part XVI)

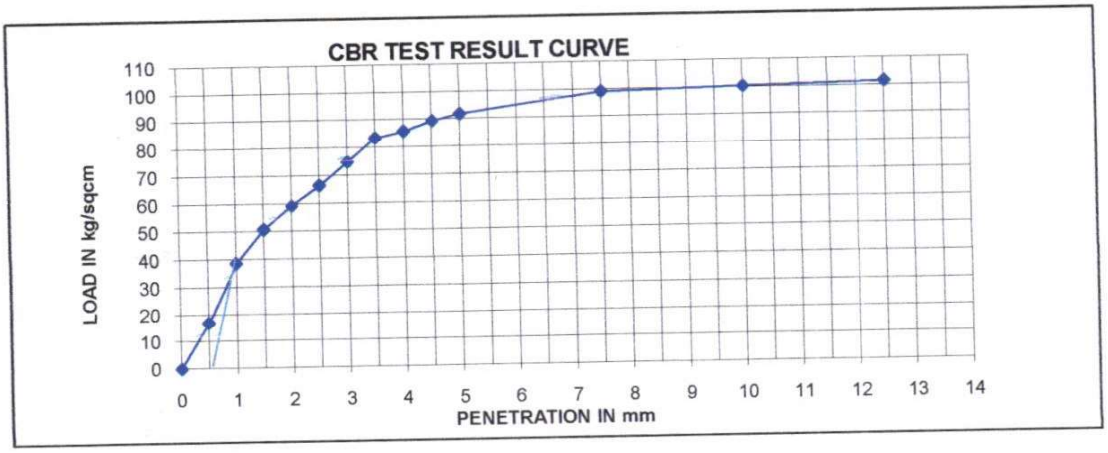
Test condition: Soaked

Nature of Sample: Compacted in 3 layers with 55 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Size of Mould = 15cm dia x 12.73 cm height Sample Taken = 6 kg

Penetration Data **Proving Ring Used: 1000kg**

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0	0				
0.5	15	16.9				
1.0	35	38.7				
1.5	46	50.8				
2.0	54	59.3				
2.5	61	66.6	73.2	1370	5.3	
3.0	68	75.0				5.3
3.5	76	83.5				
4.0	78	85.9				
4.5	81	89.5				
5.0	84	92.0	91.3	2055	4.4	
7.5	90	99.2				
10.0	91	100.4				
12.5	92	101.6				



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LOCATON -BH2

LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part XVI)

Test condition: Soaked

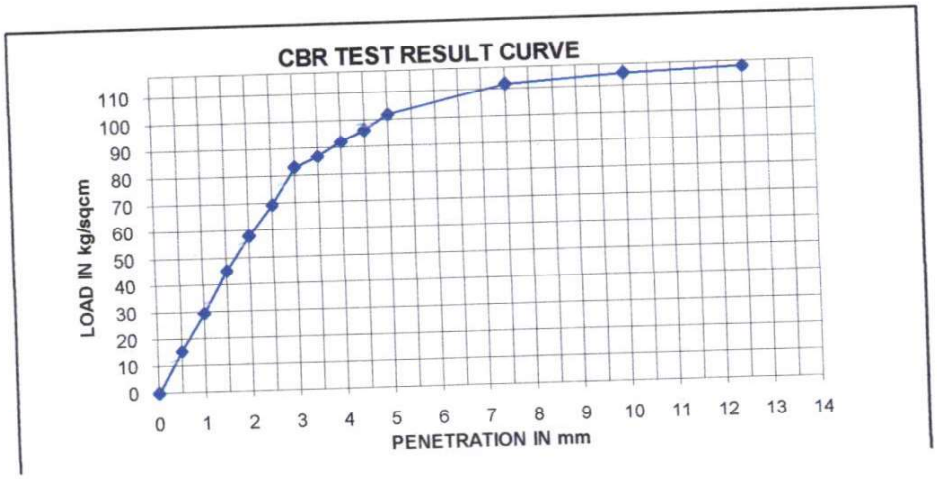
Nature of Sample: Compacted in 3 layers with 55 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Size of Mould = 15cm dia x 12.73 cm height Sample Taken = 6 kg

Proving Ring Used: 1000kg

Penetration Data

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0	0				
0.5	12.0	15.6				
1.0	22.9	29.7				
1.5	34.9	45.3				
2.0	44.7	58.1				
2.5	53.4	69.4	69.4	1370	5.1	
3.0	64.0	83.1				5.1
3.5	67.0	87.0				
4.0	71.0	92.2				
4.5	74.1	96.3				
5.0	78.5	101.9	101.9	2055	5.0	
7.5	86.1	111.9				
10.0	88.3	114.7				
12.5	89.4	116.1				



RELIANT FOUNDATIONS

Location: BH3

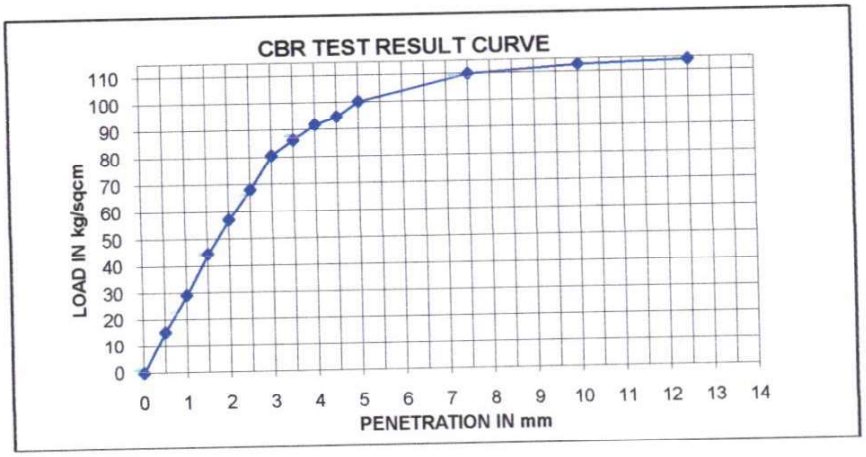
LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part XVI)

Test condition: Soaked

Nature of Sample: Compacted in 3 layers with 55 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Penetration Data **Proving Ring Used: 1000kg**

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0	0				
0.5	13	15.3				
1.0	25	29.2				
1.5	38	44.6				
2.0	48	57.1				
2.5	58	68.2	68.2	1370	5.0	
3.0	68	80.8				5.0
3.5	73	86.3				
4.0	78	91.9				
4.5	80	94.7				
5.0	85	100.3	100.3	2055	4.9	
7.5	93	110.0				
10.0	96	112.8				
12.5	97	114.2				



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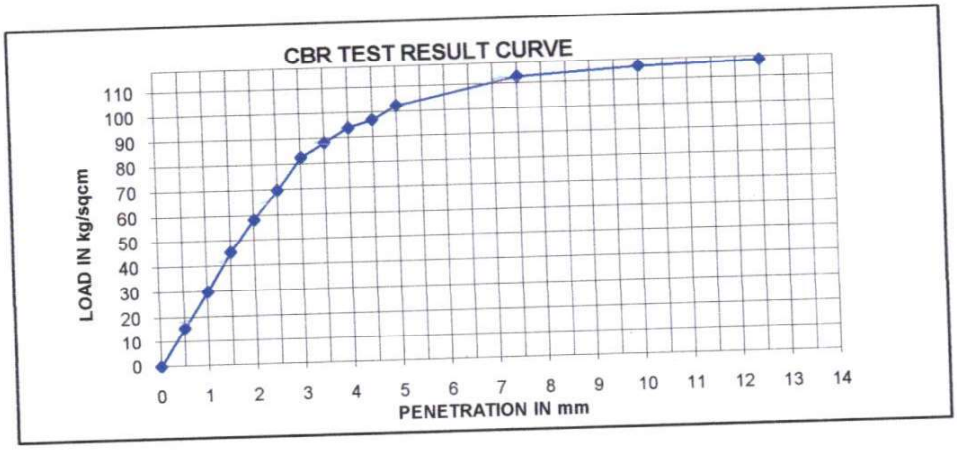
Location:-BH4

LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part XVI)

Test condition: Soaked
Nature of Sample: Compacted in 3 layers with 25 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Penetration Data **Proving Ring Used: 1000kg**

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0.0	0				
0.5	11.4	15.7				
1.0	21.8	29.9				
1.5	33.3	45.6				
2.0	42.6	58.4				
2.5	51.0	69.8	69.8	1370	5.1	
3.0	60.3	82.6				5.1
3.5	64.5	88.3				
4.0	68.6	94.0				
4.5	70.7	96.9				
5.0	74.9	102.6	102.6	2055	5.0	
7.5	82.2	112.6				
10.0	84.2	115.4				
12.5	85.3	116.8				



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Location:-BH5

LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part XVI)

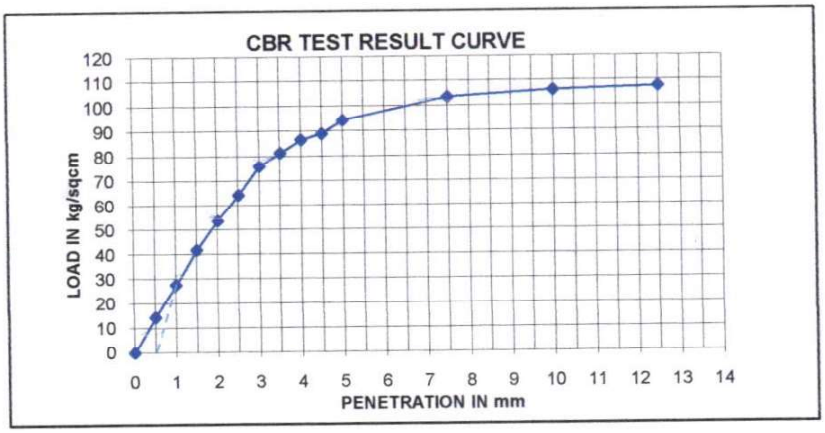
Test condition: Soaked

Nature of Sample: Compacted in 3 layers with 55 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Size of Mould = 15cm dia x 12.73 cm height Sample Taken = 6 kg

Penetration Data **Proving Ring Used: 1000kg**

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0	0				
0.5	13	14.4				
1.0	24	27.5				
1.5	36	42.0				
2.0	47	53.8				
2.5	56	64.2	77	1370	5.6	
3.0	66	76.0				5.6
3.5	71	81.3				
4.0	75	86.5				
4.5	78	89.1				
5.0	82	94.4	97	2055	4.7	
7.5	90	103.6				
10.0	92	106.2				
12.5	93	107.5				



RELIANT FOUNDATIONS PVT. LTD.

Location:-BH6

LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part XVI)

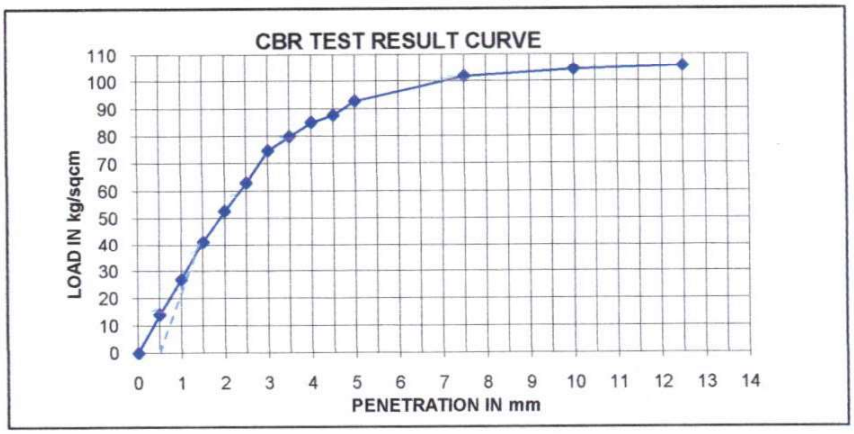
Test condition: Soaked

Nature of Sample: Compacted in 3 layers with 55 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Size of Mould = 15cm dia x 12.73 cm height Sample Taken = 6 kg

Penetration Data **Proving Ring Used: 1000kg**

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0	0				
0.5	11	14.2				
1.0	21	27.1				
1.5	32	41.3				
2.0	41	52.9				
2.5	49	63.2	77.2	1370	5.6	
3.0	58	74.8				5.6
3.5	62	80.0				
4.0	66	85.1				
4.5	68	87.7				
5.0	72	92.9	95.2	2055	4.6	
7.5	79	101.9				
10.0	81	104.5				
12.5	82	105.8				



RELIANT FOUNDATION

Location:-BH-7

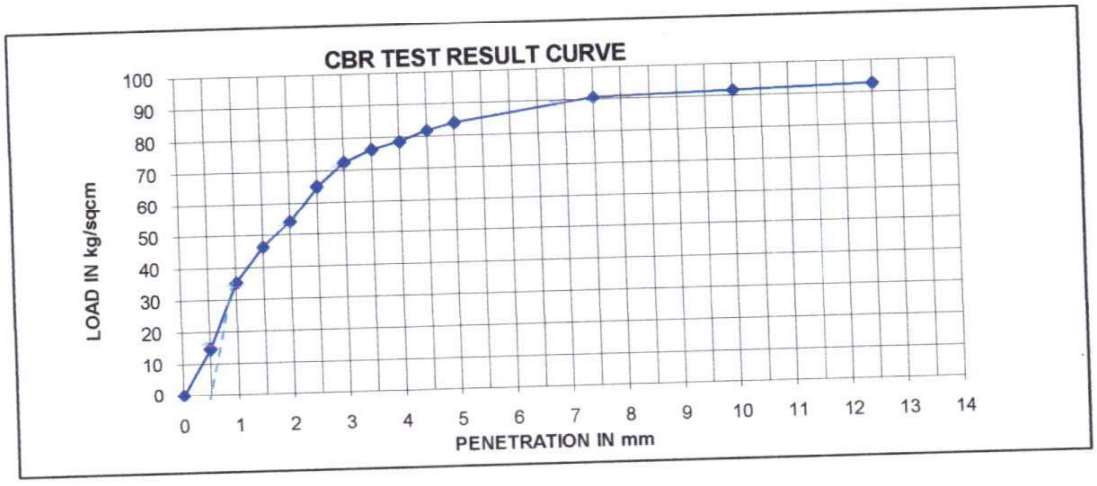
LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part XVI)

Test condition: Soaked

Nature of Sample: Compacted in 3 layers with 25 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Penetration Data **Proving Ring Used: 1000kg**

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0	0				
0.5	12	14.8				
1.0	29	35.4				
1.5	38	46.5				
2.0	44	54.2				
2.5	53	65.2	71.6	1370	5.2	
3.0	59	72.6				5.2
3.5	62	76.4				
4.0	64	78.6				
4.5	67	81.9				
5.0	68	84.1	85.6	2055	4.2	
7.5	74	90.8				
10.0	75	91.9				
12.5	76	93.0				



RELIANT FOUNDATIONS PVT LTD

Location: BH8

LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part X

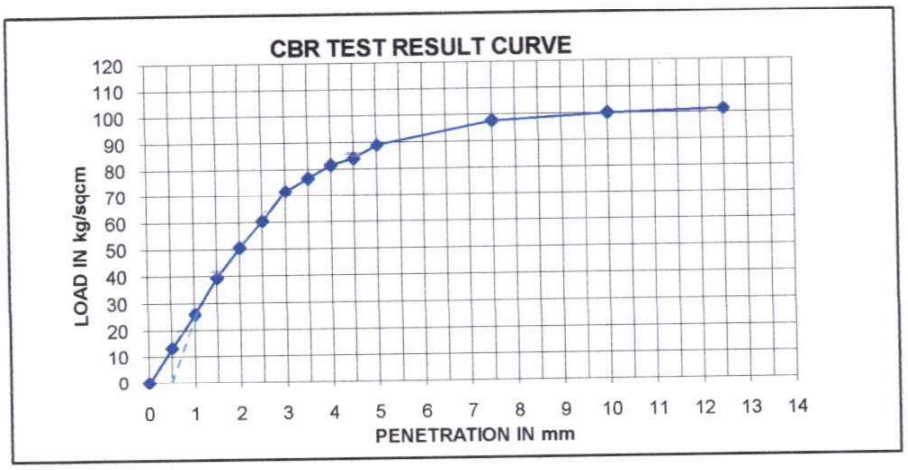
Test condition: Soaked

Nature of Sample: Compacted in 3 layers with 55 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Size of Mould = 15cm dia x 12.73 cm height Sample Taken = 6 kg

Penetration Data **Proving Ring Used: 1000kg**

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0	0				
0.5	11	13.6				
1.0	22	26.0				
1.5	33	39.6				
2.0	43	50.7				
2.5	51	60.6	70.5	1370	5.1	
3.0	60	71.8				5.1
3.5	64	76.7				
4.0	69	81.7				
4.5	71	84.2				
5.0	75	89.1	90.6	2055	4.4	
7.5	82	97.8				
10.0	84	100.2				
12.5	85	101.5				



RELIANT FOUNDATIONS PVT. LTD.

Location BH-09

LABORATORY C.B.R. TEST DATA SHEET (As per IS : 2720 part XVI)

Test condition: Soaked

Nature of Sample: Compacted in 3 layers with 55 blows of 2.6 kg rammer having 31 cm drop at OMC and 100% Proctor Density (approx)

Size of Mould = 15cm dia x 12.73 cm height Sample Taken = 6 kg

Penetration Data **Proving Ring Used: 1000kg**

Penetration (mm)	TEST LOAD (kg)		Corrected load (kg)	Standard Load (kg)	% standard load (%)	C.B.R.
	Proving ring Reading	Load				
0	0	0				
0.5	12	14.4				
1.0	23	27.5				
1.5	35	41.9				
2.0	45	53.6				
2.5	53	64.1	76	1370	5.5	
3.0	64	76.8				5.5
3.5	67	80.4				
4.0	71	85.2				
4.5	74	88.9				
5.0	78	94.2	97	2055	4.7	
7.5	86	103.3				
10.0	88	105.9				
12.5	89	107.3